GENERAL NOTES

CODES AND SPECIFICATIONS

- 1. INTERNATIONAL BUILDING CODE, 2006.
- 2. DEPARTMENT OF VETERAN AFFAIRS SEISMIC DESIGN REQUIREMENTS (H-18-8 JULY 2008)
- 3. ACI 301-99 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.
- 4. AISC-1992 CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AS MODIFIED BY THE CONSTRUCTION
- 5. ANSI/AWS STRUCTURAL WELDING CODE STEEL D1.1
- **FOUNDATIONS** 1. THE FOUNDATION DESIGN IS BASED UPON THE RECOMMENDATIONS INCLUDED IN A REPORT BY TERRACON, DATED JUNE 3,
- 2. FOUNDATION ELEVATIONS ARE ESTIMATED AND ARE FOR BIDDING PURPOSES ONLY, AND MAY VARY TO SUIT SUBSURFACE SOIL CONDITIONS.
- 3. ALL BEARING SURFACES SHALL BE UNDISTURBED, LEVEL (WITHIN 1 IN 12), AND SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE. UNLESS APPROVED OTHERWISE BY THE GEOTECHNICAL ENGINEER, ALL FOOTINGS ARE TO BE POURED NEAT (WITHOUT SIDE FORMS) WHERE SOILS PERMIT, OTHERWISE SIDES SHALL BE FORMED.
- 4. COLUMN FOOTINGS ARE DESIGNED FOR A MAXIMUM BEARING PRESSURE OF 4000 PSF ON NATURAL SOIL; WALL FOOTINGS ARE DESIGNED FOR A MAXIMUM BEARING PRESSURE OF 3000 PSF ON NATURAL SOIL. SOILS UNSUITABLE FOR SUPPORTING FOUNDATIONS SHALL BE REMOVED AS DIRECTED BY CONTRACTOR'S GEOTECHNICAL TESTING CONSULTANT, AND BACKFILLED TO DESIGN BEARING ELEVATION WITH LEAN CONCRETE.
- 5. SET COLUMN DOWELS AND ANCHOR BOLTS WITH TEMPLATE PRIOR TO CONCRETING. <u>CONCRETE</u>
- CONCRETE STRENGTHS:
- A. FOOTINGS, AND GRADE BEAMS: 3000 PSI
- B. TYPICAL CONCRETE UNLESS NOTED OTHERWISE: 4000 PSI
- C. EXTERIOR CONCRETE EXPOSED TO WEATHER U.N.O: 4500 PSI AE
- D. INTERIOR CONCRETE SLABS ON METAL DECK: 4000 PSI LW
- E. BACKFILL (LEAN) CONCRETE: 1500 PSI
- 2. PROVIDE 3/4" BEVELS AT CORNERS OF ALL COLUMNS, EDGES OF EXPOSED BEAMS AND TOP EDGES AND CORNERS OF
- 3. MAXIMUM LENGTH OF WALL POUR BETWEEN CONSTRUCTION JOINTS SHALL NOT EXCEED 120 FEET. MAXIMUM AREA OF SLAB POURS NOT TO EXCEED 10,000 SF.
- 4. JOINTS NOT INDICATED ON STRUCTURAL DRAWINGS ARE NOT PERMITTED UNLESS APPROVED BY STRUCTURAL ENGINEER.
- 5. PLACE NO PERMANENT LOAD, SUCH AS MASONRY WALLS, ON SUPPORTED SLABS UNTIL CONCRETE HAS REACHED SPECIFIED STRENGTH AND ALL SHORING HAS BEEN REMOVED.
- 6. PLACE NO OPENINGS, SLEEVES, INSERTS, ETC., IN CONCRETE WORK UNLESS CRITERIA INDICATED ON STRUCTURAL
- DRAWINGS IS MET, OR IS APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. 7. CONCRETE CONSTRUCTION TOLERANCES ARE AS SHOWN IN THE PROJECT SPECIFICATIONS.
- 8. LEDGE ANGLE INSERTS AND BOLTS:
- A. INSERTS SHALL BE WEDGE TYPE INSERTS. THE MAIN BODY OF THE INSERT SHALL HAVE AN OVERALL HEIGHT OF AT LEAST 3-7/8" AND PROVIDE A MINIMUM 1-3/4" VERTICAL ADJUSTMENT FOR 3/4" DIAMETER ASKEW HEAD BOLTS. THE INSERTS SHALL HAVE AN ATTACHED ANCHOR LOOP THAT PROJECTS AT LEAST 2-1/2" BEHIND THE BODY OF THE
- B. THE INSERT SUPPLIER SHALL CERTIFY THE INSERTS WILL SIMULTANEOUSLY SUPPORT A VERTICAL LOAD OF 1000 POUNDS AND A PULLOUT LOAD OF 1600 POUNDS WITH A SAFETY FACTOR OF 3. VERTICAL SLIP SHALL NOT EXCEED 1/16". CERTIFICATION MUST BE ACCOMPANIED BY TEST RESULTS FROM AN INDEPENDANT TESTING AGENCY.
- C. BOLTS SHALL BE 3/4" DIAMETER ASKEW HEAD BOLTS SUPPLIED BY THE INSERT MANUFACTURER. OF LENGTH
- REQUIRED TO MEET THE CAPACITY REQUIREMENTS SHOWN ABOVE, INCLUDING ALL CONCRETE CONSTRUCTION TOLERANCES, SHIMS, ETC.
- D. STEEL SHIMS SHALL BE PROVIDED AS DETAILED ON THE DRAWINGS. SHIMS SHALL BE "U-SHAPED" AND SHALL FULLY BEAR AGAINST THE CONCRETE SURFACE.
- E. INSERTS, BOLTS, NUTS, WASHERS, AND SHIMS ARE TO BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM
- F. SUBMITTALS REQUIRED BY INSERT MANUFACTURER: a. INSTALLATION INSTRUCTIONS INDICATING THE FOLLOWING:
- 1. MINIMUM AIR TEMPERATURE RESTRICTIONS, IF ANY, FOR TORQUING BOLTS. 2. SPECIFIED TORQUE TO OBTAIN A SAFETY FACTOR OF 2 AGAINST SLIP BETWEEN ANGLE, SHIMS, AND
- 3. ANY SPECIAL INSTRUCTIONS REGARDING PROPER INSTALLATION OF PROPOSED INSERT. b. TEST DATA FOR INSERT FURNISHED INDICATING SHEAR AND PULLOUT CAPACITY OF INSERT. c. CERTIFICATION AS DESCRIBED HEREIN.

REINFORCING STEEL

- ALL REINFORCING: 60 KSI YIELD
- 2. REINFORCE ALL SLABS AS FOLLOWS UNLESS OTHERWISE NOTED, FURNISH MESH IN FLAT SHEETS:
- A. SLABS ON METAL DECK: 6x6-W2.9xW2.9 (42#) WWF
- B. INTERIOR AND EXTERIOR SLABS ON GROUND: 6x6-W4.0xW4.0 (EPOXY COATED AT EXTERIOR) (58#) WWF
- 3. PROVIDE TENSION SPLICES UNLESS OTHERWISE NOTED.
- 4. CLEARANCES BETWEEN REINFORCING BARS AND CONCRETE SURFACES SHALL BE ACI MINIMUM UNLESS OTHERWISE
- 5. PROVIDE MATERIAL AND PLACEMENT OF 1 TON OF CONTINGENCY REINFORCING STEEL, SIZES OF BARS TO BE MIXED. BARS ARE TO BE CUT, BENT, AND PLACED AS DIRECTED BY THE ENGINEER.

STRUCTURAL STEEL MATERIAL:

- A. ROLLED SHAPES AND PLATES: ASTM A-36
- B. WIDE FLANGES: ASTM A-992, (50 KSI).
- C. HOLLOW STRUCTURAL SHAPES (HSS): ASTM A-500, GRADE B
- D. STEEL PIPES: ASTM 53, GRADE B
- E. HEADED SHEAR STUDS: COMPLY WITH AWS D1.1
- 2. FIELD BOLTS: ASTM A-325, 3/4" DIAMETER, TENSION-CONTROL, UNLESS NOTED OTHERWISE
- 3. ANCHOR BOLTS: ASTM F1554, UNLESS NOTED OTHERWISE.
- 4. ALL STEEL EXPOSED TO WEATHER INCLUDING EXTERIOR WALL LINTELS SHALL BE HOT DIPPED GALVANIZED AS DESCRIBED IN PROJECT SPECIFICATIONS.
- 5. FABRICATOR TO DESIGN CONNECTION NOT DETAILED:
- A. CONNECTIONS ARE TO BE DESIGNED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, THE AISC LRFD SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, THE AISC MANUAL, AND THE STRUCTURAL STEEL FRAMING PLANS, NOTES AND DETAILS OF THESE DRAWINGS.
- B. CONNECTION CONFIGURATIONS INDICATED ON THE PLANS, NOTES AND DETAILS REPRESENT THE DESIGN INTENT ITEMS SPECIFICALLY INDICATED: WELDS, STIFFENERS, BRACES, ETC. MUST BE PROVIDED AT MINIMUM AS SHOWN. ADDITIONAL DESIGN AND DETAILING OF CONNECTION, INCLUDING CONSIDERATION OF MEMBER THICKNESS, HOLES, CUTS, COPES, AND THE EFFECTS OF CONCENTRATED FORCES SHALL BE PROVIDED BY THE CONTRACTOR.
- C. UNLESS SPECIFIC REACTIONS, MOMENTS, SHEARS, AND AXIAL FORCES ARE INDICATED, DESIGN BEAM CONNECTIONS FOR REACTIONS DUE TO THE MAXIMUM UNIFORM LOAD THE BEAM CAN SUPPORT AT ITS SPAN, AS SHOWN IN THE AISC MANUAL FOR SPECIFIED YIELD STRENGTH.
- 6. PROVIDE MATERIAL AND PLACEMENT OF 1 TON OF CONTINGENCY STRUCTURAL STEEL (SHAPE AND SIZE TO BE DETERMINED). PLACE AS DIRECTED BY THE STRUCTURAL ENGINEER.

METAL FLOOR AND ROOF DECK

1. ALL WORK SHALL CONFORM TO STEEL DECK INSTITUTE (SDI) SPECIFICATIONS FOR COMPOSITE STEEL FLOOR DECK, NON-COMPOSITE STEEL FORM DECK, STEEL ROOF DECK.

- 2. MATERIAL (GALVANIZED STEEL PER PROJECT SPECIFICATIONS):
 - 2" DEEP, 20 GAGE (MINIMUM) COMPOSITE METAL FLOOR DECK.
- 3. ATTACHMENT OF FLOOR DECK: WELD TO ALL SUPPORTS WITH 5/8" DIAMETER FUSION WELDS AT 6" AT ALL END LAPS, AND AT 12" ON CENTER AT ALL INTERMEDIATE SUPPORTS AND EDGES PARALLEL WITH DECK FLUTES. PROVIDE SCREWED SIDE LAP FASTENERS AT 36" MAXIMUM SPACING.
- 1. FIELD VERIFY EXISTING DIMENSIONS AND ELEVATIONS WHICH AFFECT FABRICATION PRIOR TO SUBMITTAL OF SHOP
- 2. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL EMBEDDED ITEMS, SLEEVES, FLOOR PITCHES, FILLS, AND DEPRESSIONS.

3. STRUCTURAL FRAMING PLANS ARE TYPICALLY DRAWN AS REFLECTED PLANS SHOWING BEAMS, WALLS, AND COLUMNS ON

- THE UNDERSIDE OF THE SLAB SHOWN. 4. ALL FRAMING MEMBERS PROVIDED FOR MECHANICAL TRADES, ELEVATOR SUPPORT BEAMS, LINTELS, ROOF OPENINGS,
- ETC., ARE FOR BIDDING PURPOSES ONLY. SUBMIT MANUFACTURER'S DATA FOR THE PROPOSED EQUIPMENT TO STRUCTURAL ENGINEER PRIOR TO SUBMITTAL OF SHOP DRAWINGS FOR VERIFICATION OR REDESIGN OF SUPPORTS.
- 1. BRACE ENTIRE STRUCTURE AS REQUIRED TO MAINTAIN STABILITY UNTIL COMPLETE AND FUNCTIONING AS THE DESIGNED
- 2. DO NOT BACKFILL FOUNDATION WALLS SPANNING BETWEEN BASEMENT SLABS AND STRUCTURAL FLOORS UNTIL SUPPORTING SLABS ARE IN PLACE.
- 3. VERIFY EXACT SIZE AND LOCATION OF ALL WALL, FLOOR, AND ROOF OPENINGS PRIOR TO SUBMISSION OF SHOP DRAWINGS. SHOW ALL OPENINGS ON SHOP DRAWINGS.
- 4. PLACEMENT OF CONDUITS IN CONCRETE SHALL ADHERE TO THE FOLLOWING:
- A. OUTSIDE DIAMETER OF CONDUITS SHALL BE 1" OR LESS WHERE EMBEDDED IN SLABS AND JOISTS, AND 1 1/2" OR LESS WHERE EMBEDDED IN BEAMS, WALLS, OR COLUMNS.

B. BUNCHING OF CONDUITS IS NOT PERMITTED. CONDUITS SHALL BE SPACED 3 DIAMETERS OR GREATER ON

- C. LONGITUDINAL PLACEMENT OF CONDUITS IN INDIVIDUAL JOISTS AND BEAMS SHALL BE LIMITED TO ONE CONDUIT
- PER JOIST AND THREE CONDUITS PER BEAM.
- D. MINIMUM CONCRETE COVER ON CONDUITS SHALL BE 1 1/2".
- E. ALUMINUM CONDUITS, PIPES, OR SLEEVES ARE NOT PERMITTED.
- MECHANICAL LOADS SUPPORTED FROM STRUCTURE
- 1. LOADS ARE TO BE DISTRIBUTED TO THE STRUCTURE IN A MANNER THAT DOES NOT EXCEED THE LOAD ALLOWANCES NOTED UNDER DESIGN LOADS. ANCHORS ARE TO HAVE AN ULTIMATE SAFETY FACTOR OF AT LEAST 4.0.
- 2. LOADS SUPPORTED BY WIDE FLANGE BEAMS ARE TO BE APPLIED IN A MANNER WHICH DOES NOT CAUSE TORSION ABOUT THE LONGITUDINAL AXIS OF THE BEAM.
- 1. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING
- 2. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF ADEQUACY OF CONTRACTOR'S SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION

<u>DESIGN LOADS</u>

- SUPERIMPOSED GRAVITY LOADS
- TYPICAL FLOOR
- A. LIVE LOAD:
- TYPICAL: 100 PSF
- B. CEILING AND MECHANICAL ALLOWANCE: 15 PSF
- ATTIC LEVEL
- A. LIVE LOAD: 100 PSF, INCLUDING ROOF LIVE LOAD
- B. CEILING AND MECHANICAL ALLOWANCE: 15 PSF
- C. ROOFING MATERIALS ALLOWANCE: 10 PSF
- D. LGMF ALLOWANCE: 15 PSF INDICATED LIVE LOADS ARE UNREDUCED
- SNOW LOAD
- GROUND SNOW LOAD (Pg) = 20 PSF
- SNOW EXPOSURE FACTOR (Ce) = 1.0 SNOW LOAD IMPORTANCE FACTOR (I) = 1.2
- WIND LOAD
- 3-SECOND GUST WIND SPEED = 90 MPH WIND LOAD IMPORTANCE FACTOR (I) = 1.15
- WIND EXPOSURE: B
- EARTHQUAKE DESIGN DATA DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS, Sds = 0.17
- DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1-SECOND PERIODS, Sd1 = 0.10 SITE CLASS = D

CONSTRUED AS SPECIAL INSPECTION.

- SITE COEFFICIENT, Fa = 1.6 SEISMIC DESIGN CATEGORY = C OCCUPANCY CATEGORY = IV
- BASIC STRUCTURAL SYSTEM AND SEISMIC RESISTING SYSTEM:
- STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE. RESPONSE MODIFICATION FACTOR (R) = 3
- ARCHITECTURAL DOCUMENTS. QUALITY ASSURANCE

1. THE CONTRACTOR WILL EMPLOY QUALIFIED INSPECTORS TO PERFORM INSPECTIONS IN ACCORDANCE WITH

1. ALL METAL STRUCTURAL FRAMING REQUIRES SPRAY-ON FIREPROOFING OR FIRERATED ENCLOSURE. REFER TO

SPECIFICATION SECTION 01 45 29 TESTING LABORATORY SERVICES. 2. THE STRUCTURAL ENGINEER WILL GENERALLY REVIEW THE PROGRESS OF THE WORK, BUT HIS REVIEW SHALL NOT BE

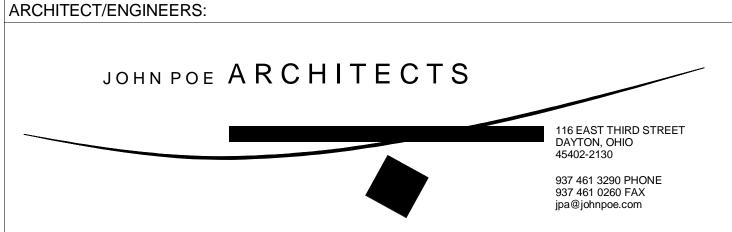
CONSULTANTS:

9/6/13

Bid Documents

Cincinnati, Ohio 45202 Phone: 513.241.3222 www.thpltd.com





GENERAL NOTES

Drawing Title

Approved: Project Director

Renovate CLC **Building 211 West**

Chillicothe, Ohio

Building Number 211 Drawing Number

Project No.

VA Project No. 538-107

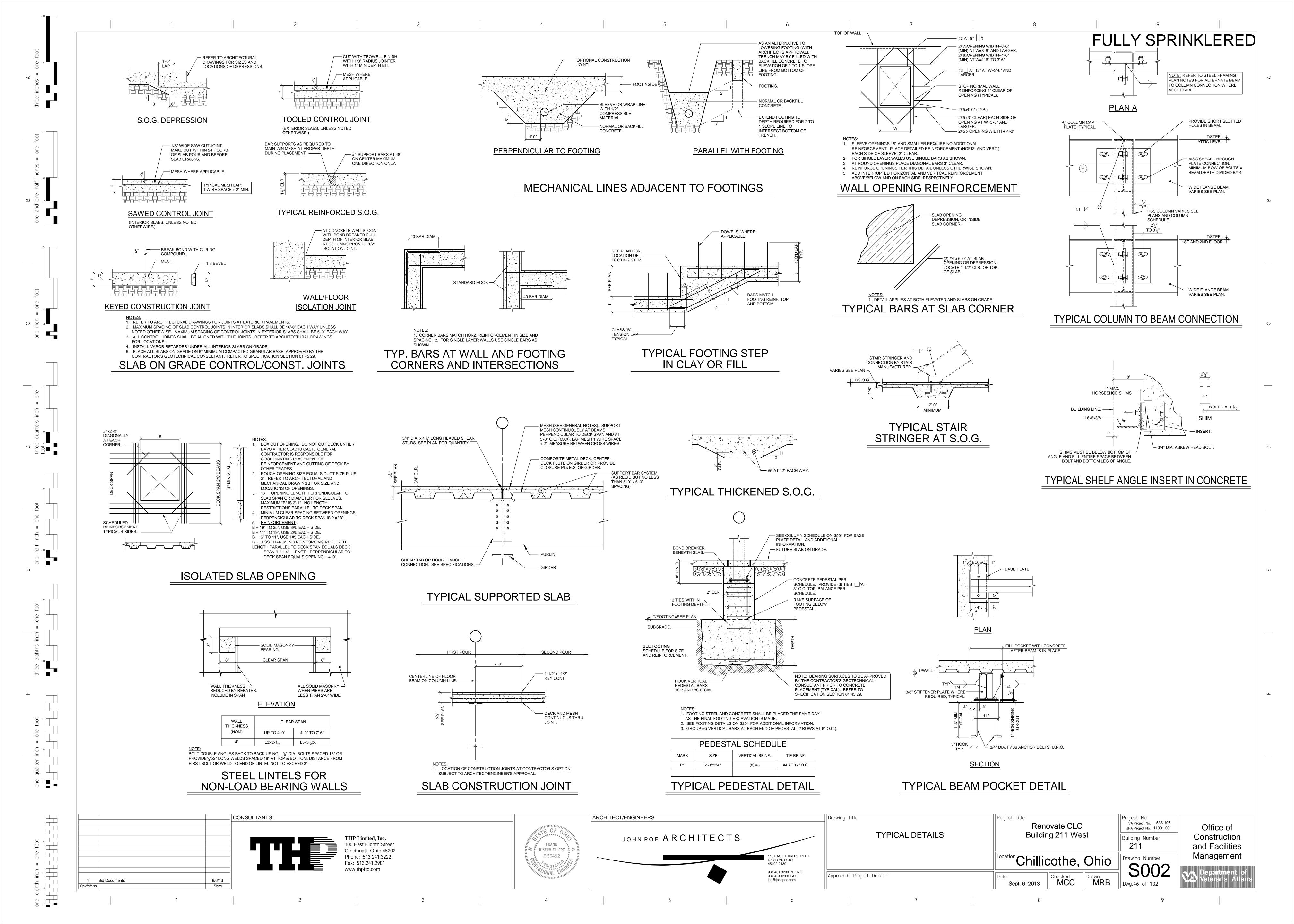
JPA Project No. 11001.00

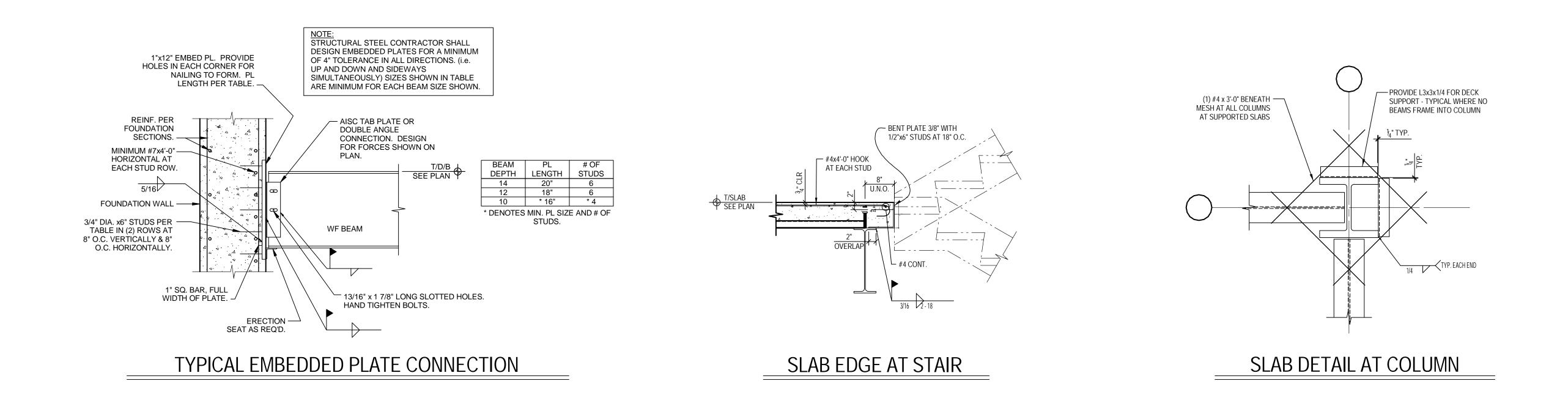
and Facilities Management

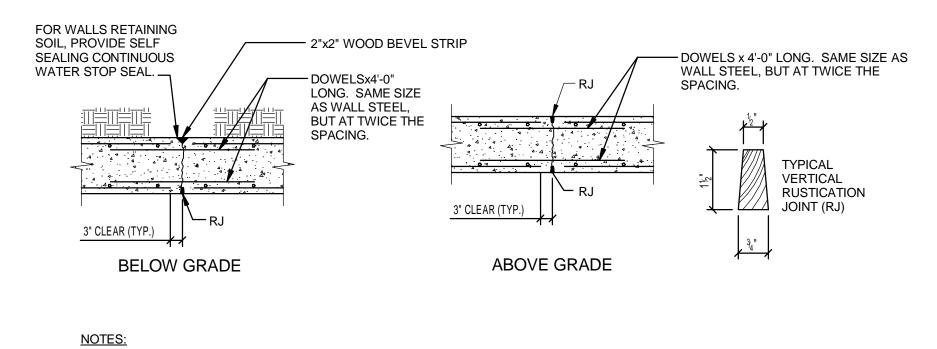
Project Title

Office of

Construction



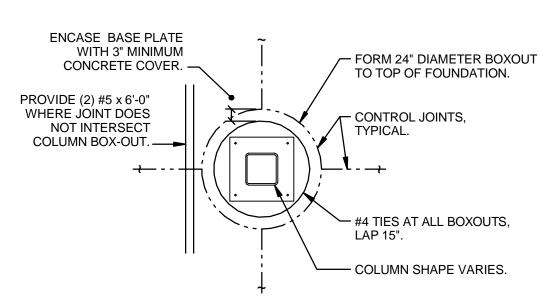




- NOTES:

 1. FORM TIES TO BE WITHIN 6" OF EITHER SIDE OF A CONSTRUCTION JOINT.

 2. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR WATERPROOFING REQUIREMENTS.
- DETAIL CONTROL JOINTS ON SHOP DRAWINGS.
 LOCATE CONSTRUCTION JOINTS AT CONTROL JOINTS. PROVIDE CONTINUOUS VERTICAL 2x4 KEY
- IN CONSTRUCTION JOINTS. 5. MAXIMUM HORIZONTAL SPACING BETWEEN CONTROL JOINTS SHALL NOT EXCEED 20'-0" FOR BASEMENT WALLS AND 16'-0' FOR RETAINING WALLS.
- VERTICAL WALL CONTROL AND CONSTRUCTION JOINTS

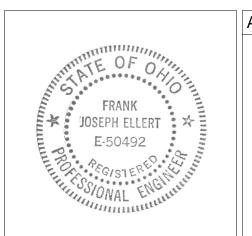


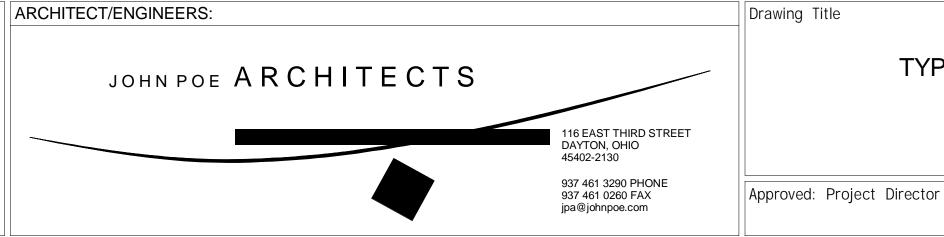
TYPICAL COLUMN BOXOUT

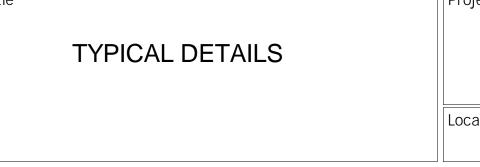
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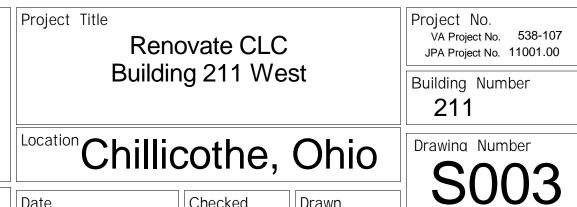
CONSULTANTS:

9/6/13 Date











one- eighth inch = one foot

1 Bid Documents

DRAWING NOTES:

1. REFER TO S001 SERIES FOR GENERAL NOTES AND TYPICAL DETAILS.

2. REFER TO S200 SERIES FOR FOUNDATION DETAILS.

3. ELEVATION: 100'-0" = 703.15'

KEY NOTES: X

1. REFER TO ARCH. FOR WALL OPENINGS.

2. PROVIDE CONTROL JOINTS IN SLAB ON GRADE AT 16'-0" MAX. EACH WAY, U.N.O.

3. CONNECT PEDESTALS WITH 12" WIDE CONCRETE WALL. REINFORCE WALL WITH #5 AT 12" E.W., E.F. T/WALL = T/PEDESTAL. DOWELS INTO TOP OF FOOTING TO MATCH VERTICAL WALL STEEL.

4. REFER TO MECHANICAL DRAWINGS FOR LOCATION OF BREAK IN SLAB ALLOWING PLACEMENT OF FUTURE SANITARY LINE. PROVIDE KEYED CONSTRUCTION JOINT FOR FUTURE SLAB PLACEMENT. 5. PROVIDE 12" THICKENED SLAB BELOW STAIR LANDING SUPPORT

POST. REFER TO TYPICAL DETAIL ON S002. COORDINATE LOCATION WITH STAIR MANUFACTURER. 6. #4 DOWELS AT 18" O.C. EPOXY EMBED 6" INTO EXISTING CONCRETE. INSTALL WATERSTOP RX BEAD BETWEEN NEW AND EXISTING

7. THICKEN S.O.G. BELOW COLUMN FOR A 2'-0"x2'-0" AREA, TOTAL DEPTH=24".

8. ADD (2) #8 TO SLAB. EXTEND 2'-0" BEYOND OPENING EDGE. 9. DO NOT DAMAGE EXISTING BASEMENT WALL OR FOUNDATIONS TO REMAIN WHILE REMOVING PORTIONS REQUIRED IN THE ARCHITECTURAL

LEGEND:

DRAWINGS.

BASEMENT WALLS.

INDICATES WALL CONTROL JOINT

INDICATES FOOTING STEP. REFER TO TYPICAL DETAIL ON S002.

- INDICATES FOOTING MARK. REFER TO SCHEDULE BELOW.

INDICATES TYPE OF PEDESTAL.

XXX'-XX" INDICATES TOP OF FOOTING ELEVATION. INDICATES TOP OF PEDESTAL ELEVATION.

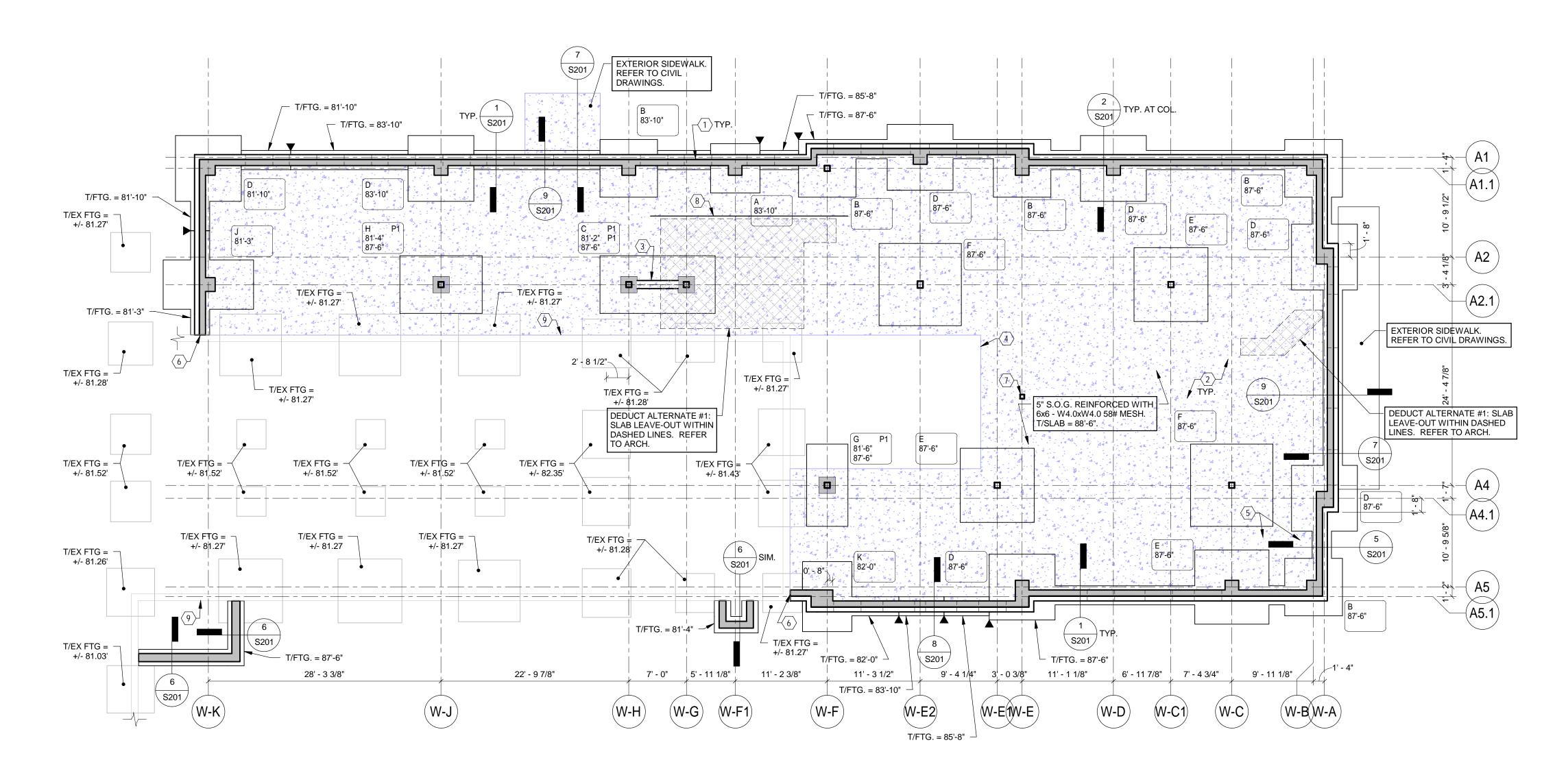
> FOOTING SCHEDULE MARK SIZE THICKNESS REINFORCING 7'x7' (7) #6 EW (7) #6 LONG С 7'x14' (14) #6 SHORT D (7) #7 EW 8'x8' (12) #7 EW 9'x9' 10'x10' (13) #7 EW (10) #6 LONG G 5'x10' (8) #6 SHORT Н 7'x10' (9) #7 SHORT (10) #6 LONG (8) #6 SHORT

> > (8) #6 LONG (9) #6 SHORT

6'x11'

6'x8'-6"

K



FOUNDATION PLAN

1/8" = 1'-0" NOTE:
FIELD VERIFY EXISTING FOOTING LOCATIONS. LOCATIONS

IF DEVIATION IS MORE THAN 0'-9".

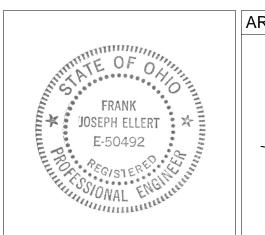
SHOWN ARE INTERPRETED FROM THE ORIGINAL BUILDING DRAWINGS AND MAY VARY FROM ACTUAL CONDITIONS. NEW FOOTINGS ADJACENT TO EXISTING FOOTING SHOULD BEAR AT SAME ELEVATION AS EXISTING. NOTIFY ENGINEER

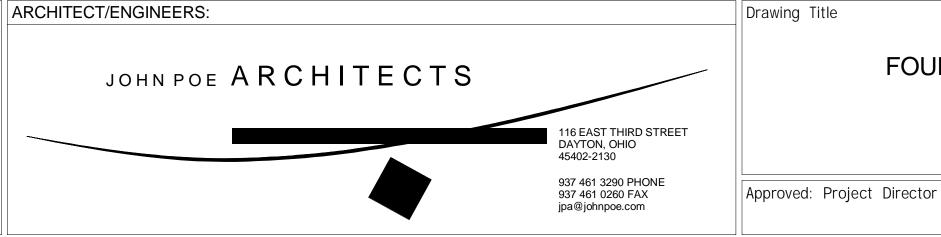
> EXISTING BUILDING N KeyPlan
> Scale: N.T.S.

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9/6/13 Date

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Project Title Renovate CLC FOUNDATION PLAN **Building 211 West**

Chillicothe, Ohio

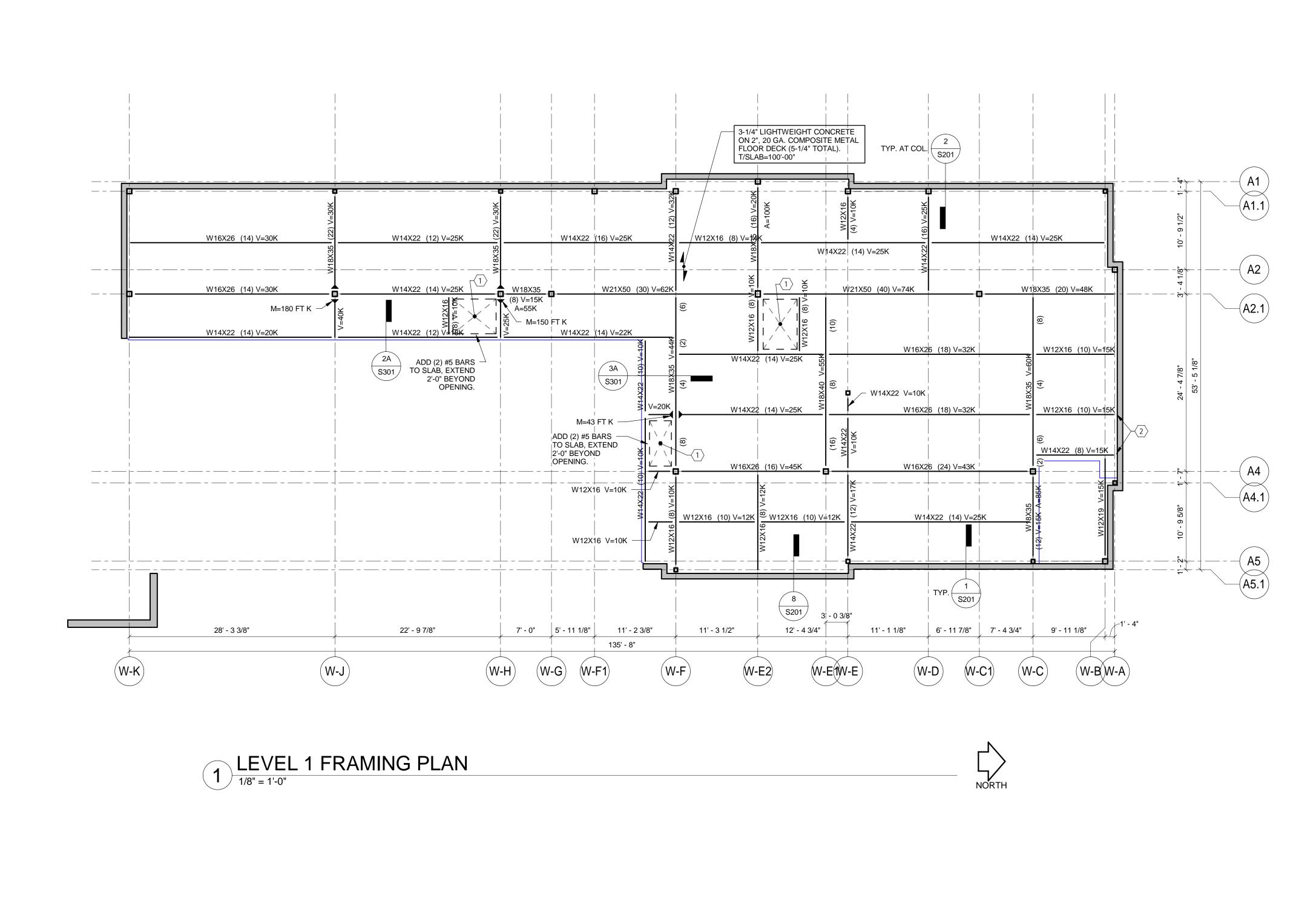
Project No.

VA Project No. 538-107

JPA Project No. 11001.00 Building Number 211 Drawing Number

S100

Office of Construction and Facilities Management



STEEL FRAMING NOTES:

1. UNDIMENSIONED MEMBERS TYPICALLY ARE EQUALLY SPACED BETWEEN DIMENSIONED MEMBERS.

2. COORDINATE SIZES AND LOCATIONS OF ALL FLOOR OPENINGS/SLEEVES WITH ARCHITECTURAL AND

MECHANICAL REQUIREMENTS. SEE TYPICAL FLOOR OPENING DETAILS.

3 FABRICATOR TO COMPLETE THE DESIGN OF ALL CONNECTIONS. SEE GENERAL NOTES AND PROJECT

3. FABRICATOR TO COMPLETE THE DESIGN OF ALL CONNECTIONS. SEE GENERAL NOTES AND PROJECT SPECIFICATIONS.

4. SHEAR CONNECTIONS AT GIRDERS TO COLUMNS ARE TO BE SHEAR THROUGH-PLATE CONNECTIONS. OTHER BEAM TO COLUMN CONNECTIONS CAN BE AISC SINGLE PLATE SHEAR CONNECTIONS TO COLUMNS (SEE NOTE 5) U.N.O. WITH THE MAXIMUM ROWS OF BOLTS = BEAM DEPTH IN INCHES DIVIDED BY 4 SPACED 3" MINIMUM O.C.

5. UNLESS INDICATED OTHERWISE SHEAR CONNECTIONS AT PURLINS TO GIRDERS MAY BE AISC SINGLE PLATE SHEAR CONNECTIONS WITH A MAXIMUM ECCENTRICITY OF 3" AND MINIMUM ROWS OF BOLTS = BEAM DEPTH IN INCHES DIVIDED BY 4, SPACED 3" MINIMUM O.C. SINGLE PLATE SHEAR CONNECTIONS SHALL HAVE HORIZONTAL SHORT SLOTTED HOLES TO THE SUPPORTED BEAM, STANDARD HOLES TO THE SUPPORTING GIRDER AND ONE SIDE MAY BE SHOP WELDED. AISC SINGLE ANGLE SHEAR CONNECTIONS MAY ALSO BE UTILIZED AT PURLIN TO GIRDER SHEAR CONNECTIONS.

6. FORCES INDICATED ON THE DRAWINGS ARE FACTORED FOR LRFD DESIGN. DESIGN BEAM CONNECTION FOR FORCES INDICATED ON THE PLANS. BRACING ELEVATIONS AND DETAILS MAY ALSO SHOW REQUIRED FORCES WHICH ARE ADDITIVE. FORCES ARE INDICATED AS:

A = AXIAL END REACTION, KIPS V = VERTICAL END REACTION, KIPS. M = BENDING MOMENT KIP FT.

7. INDICATES DIRECTION OF DECK SPAN.

8. SCHEDULE ERECTION SEQUENCE AND/OR PROVIDE ADDITIONAL TEMPORARY VERTICAL BRACING SO THAT ALL PORTIONS OF THE STRUCTURAL FRAME ARE BRACED FOR LATERAL LOADS.

FOR BRICK LEDGE INFORMATION SEE S201.
 COMPOSITE SHEAR CONNECTORS TO BE ¾" DIA. HEADED STUDS. 4 1/2" LONG U.N.C

10. COMPOSITE SHEAR CONNECTORS TO BE 3/4" DIA. HEADED STUDS, 4 1/2" LONG U.N.O. PROVIDE ADDITIONAL STUDS SO THAT THE MAXIMUM INSTALLED SPACING DOES NOT EXCEED 24" O.C., REGARDLESS OF QUANTITY INDICATED. IF NONE ARE INDICATED, PROVIDE AT 24" O.C. ON ALL BEAMS OVER 22 FEET LONG WITH CONCRETE SLABS.

11. W18X35 (16) - A SINGLE NUMBER GIVEN WITH A BEAM SIZE INDICATES TOTAL NUMBER OF STUDS

EVENLY SPACED ALONG THE BEAM OR GIRDER. WHERE BEAMS FRAME PERPENDICULAR, SPACE QUANTITIES OF STUDS OUT EVENLY ALONG GIRDER BETWEEN INTERSECTING PURLINS/COLUMNS.

12. SCHEDULE ERECTION SEQUENCE AND/OR PROVIDE ADDITIONAL TEMPORARY VERTICAL BRACING SO THAT ALL PORTIONS OF THE STRUCTURAL FRAME ARE BRACED FOR LATERAL LOADS. HORIZONTAL DECK DIAPHRAGMS ARE TO BE COMPLETED BETWEEN STABILIZING BRACES PRIOR TO CONCRETE

13. FLOOR DECK IS TO BE CONTINUOUSLY SUPPORTED AT EACH END OF DECK SPANS. PROVIDE ADDITIONAL L3X3X1/4 DECK SUPPORT ANGLES WHERE REQUIRED.

14. ALL FRAMING CONNECTIONS, GROUTING OF COLUMN BASE PLATES, AND INSPECTIONS TO BE COMPLETED PRIOR TO CASTING SLAB.

15. SLABS ARE TO BE CAST LEVEL, WITH MINIMUM THICKNESS SHOWN ON DRAWINGS. PROVIDE AN ASSUMED EXTRA 3/4" OF CONCRETE TO ACCOUNT FOR METAL DECK AND BEAM DEFLECTION. PURLINS AND GIRDERS ARE UNSHORED DURING SLAB PLACEMENT. AFTER PLACEMENT, FINISH SLABS TO REQUIRED TOLERANCES, REFER TO SPECIFICATIONS.

16. CONTRACTOR IS RESPONSIBLE TO PROVIDE A FINISHED SLAB EDGE WHICH IS STRAIGHT AND TRUE AND ACCURATELY LOCATED RELATIVE TO FINISH WALL LINES. GAGES SHOWN FOR CLOSURE ANGLES ARE THE MINIMUM REQUIRED. HEAVIER GAGE, STRAPS AND/OR A RETURN LIP AT TOP OF ANGLE MAY BE USED AT CONTRACTORS OPTION. WELD CLOSURE ANGLES TO DECK/BEAM AT 12" MAX. O.C. OR AS REQUIRED TO PREVENT TORSIONAL DEFLECTION OF ANGLE.

17. ALL ANGLES/MEMBERS SUPPORTING THE BUILDING FAÇADE, ARCHITECTURAL WALLS OF MASONRY OR GLASS, AND ITEMS NOTED "MASONRY SUPPORT" OR "CFMF SUPPORT" ARE CONSIDERED 'ADJUSTABLE ITEMS' AND ARE TO BE LOCATED WITH 3/8" OF THEIR DESIGN VERTICAL AND HORIZONTAL LOCATION RELATIVE TO THE ESTABLISHED FINISH WALL LINE, FINISH FLOOR LINE, AND MASONRY COURSING.

DRAWING NOTES:

PLACEMENT.

1. REFER TO S001 SERIES DRAWINGS FOR GENERAL NOTES AND TYPICAL DETAILS.

2. REFER TO S300 SERIES DRAWINGS FOR FRAMING DETAILS.

3. REFER TO DRAWING S501 FOR STEEL COLUMN SCHEDULE.

4. REFER TO DRAWING S401 FOR VERTICAL BRACING ELEVATIONS AND DETAILS.

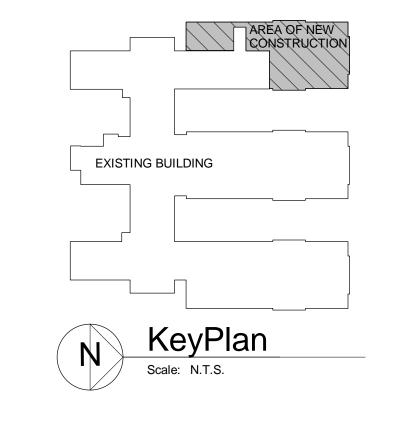
5. REFER TO DRAWING S002 FOR TYPICAL BEAM POCKET DETAIL.

6. COORDINATE DUCT AND FIRE DAMPER SLEEVED OPENINGS WITH ARCHITECTURAL AND MECHANICAL REQUIREMENTS.

KEY NOTES: X

1. PROPOSED DAMPER/DUCT LOCATION. OPENING LOCATION AND SLAB FRAMING MUST BE COORDINATED WITH STRUCTURAL ENGINEER PRIOR TO SLAB PLACEMENT.

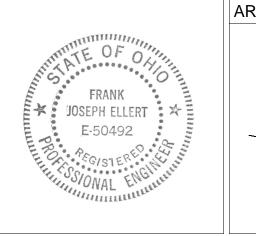
2. EMBED PLATE CONNECTION ABOVE DOORWAY. REFER TO TYPICAL EMBEDDED PLATE DETAIL ON S003.

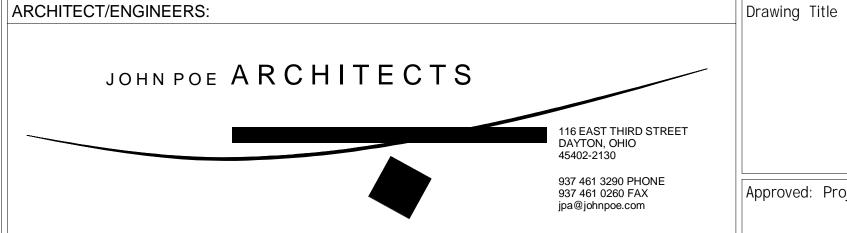




one-eighth inch = one foot

1 Bid Documents

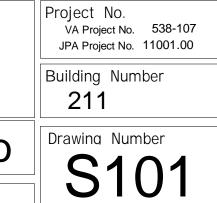








Renovate CLC



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STEEL FRAMING NOTES:

- 1. UNDIMENSIONED MEMBERS TYPICALLY ARE EQUALLY SPACED BETWEEN DIMENSIONED MEMBERS.
- 2. COORDINATE SIZES AND LOCATIONS OF ALL FLOOR OPENINGS/SLEEVES WITH ARCHITECTURAL AND MECHANICAL REQUIREMENTS. SEE TYPICAL FLOOR OPENING DETAILS.
- 3. FABRICATOR TO COMPLETE THE DESIGN OF ALL CONNECTIONS. SEE GENERAL NOTES AND PROJECT SPECIFICATIONS.
- 4. SHEAR CONNECTIONS AT GIRDERS TO COLUMNS ARE TO BE SHEAR THROUGH-PLATE CONNECTIONS. OTHER BEAM TO COLUMN CONNECTIONS CAN BE AISC SINGLE PLATE SHEAR CONNECTIONS TO COLUMNS (SEE NOTE 5) U.N.O. WITH THE MAXIMUM ROWS OF BOLTS = BEAM DEPTH IN INCHES DIVIDED BY 4 SPACED 3" MINIMUM O.C.
- 5. UNLESS INDICATED OTHERWISE SHEAR CONNECTIONS AT PURLINS TO GIRDERS MAY BE AISC SINGLE PLATE SHEAR CONNECTIONS WITH A MAXIMUM ECCENTRICITY OF 3" AND MINIMUM ROWS OF BOLTS = BEAM DEPTH IN INCHES DIVIDED BY 4, SPACED 3" MINIMUM O.C. SINGLE PLATE SHEAR CONNECTIONS SHALL HAVE HORIZONTAL SHORT SLOTTED HOLES TO THE SUPPORTED BEAM, STANDARD HOLES TO THE SUPPORTING GIRDER AND ONE SIDE MAY BE SHOP WELDED. AISC SINGLE ANGLE SHEAR CONNECTIONS MAY ALSO BE UTILIZED AT PURLIN TO GIRDER SHEAR CONNECTIONS.
- 6. FORCES INDICATED ON THE DRAWINGS ARE FACTORED FOR LRFD DESIGN. DESIGN BEAM CONNECTION FOR FORCES INDICATED ON THE PLANS. BRACING ELEVATIONS AND DETAILS MAY ALSO SHOW REQUIRED FORCES WHICH ARE ADDITIVE. FORCES ARE INDICATED AS:

A = AXIAL END REACTION, KIPS V = VERTICAL END REACTION, KIPS. M = BENDING MOMENT KIP FT.

- 7. INDICATES DIRECTION OF DECK SPAN.
- 8. SCHEDULE ERECTION SEQUENCE AND/OR PROVIDE ADDITIONAL TEMPORARY VERTICAL BRACING SO THAT ALL PORTIONS OF THE STRUCTURAL FRAME ARE BRACED FOR LATERAL LOADS.
- 9. FOR BRICK LEDGE INFORMATION SEE S201. 10. COMPOSITE SHEAR CONNECTORS TO BE 3/4" DIA. HEADED STUDS, 4 1/2" LONG U.N.O. PROVIDE
- ADDITIONAL STUDS SO THAT THE MAXIMUM INSTALLED SPACING DOES NOT EXCEED 24" O.C., REGARDLESS OF QUANTITY INDICATED. IF NONE ARE INDICATED, PROVIDE AT 24" O.C. ON ALL BEAMS OVER 22 FEET LONG WITH CONCRETE SLABS.
- 11. W18X35 (16) A SINGLE NUMBER GIVEN WITH A BEAM SIZE INDICATES TOTAL NUMBER OF STUDS EVENLY SPACED ALONG THE BEAM OR GIRDER. WHERE BEAMS FRAME PERPENDICULAR, SPACE QUANTITIES OF STUDS OUT EVENLY ALONG GIRDER BETWEEN INTERSECTING PURLINS/COLUMNS.
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- 13. FLOOR DECK IS TO BE CONTINUOUSLY SUPPORTED AT EACH END OF DECK SPANS. PROVIDE ADDITIONAL L3X3X1/4 DECK SUPPORT ANGLES WHERE REQUIRED.
- 14. ALL FRAMING CONNECTIONS, GROUTING OF COLUMN BASE PLATES, AND INSPECTIONS TO BE COMPLETED PRIOR TO CASTING SLAB. 15. SLABS ARE TO BE CAST LEVEL, WITH MINIMUM THICKNESS SHOWN ON DRAWINGS. PROVIDE AN

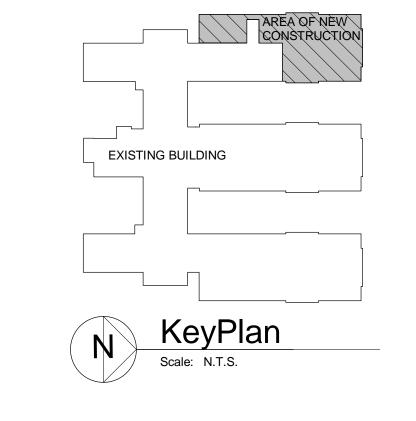
ASSUMED EXTRA 3/4" OF CONCRETE TO ACCOUNT FOR METAL DECK AND BEAM DEFLECTION. PURLINS

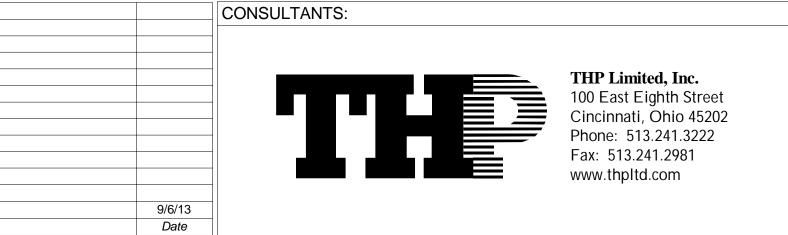
- AND GIRDERS ARE UNSHORED DURING SLAB PLACEMENT. AFTER PLACEMENT, FINISH SLABS TO REQUIRED TOLERANCES, REFER TO SPECIFICATIONS. 16. CONTRACTOR IS RESPONSIBLE TO PROVIDE A FINISHED SLAB EDGE WHICH IS STRAIGHT AND TRUE AND ACCURATELY LOCATED RELATIVE TO FINISH WALL LINES. GAGES SHOWN FOR CLOSURE ANGLES ARE THE MINIMUM REQUIRED. HEAVIER GAGE, STRAPS AND/OR A RETURN LIP AT TOP OF ANGLE MAY BE
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DRAWING NOTES:

- 1. REFER TO S001 SERIES DRAWINGS FOR GENERAL NOTES AND TYPICAL DETAILS.
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- 3. REFER TO DRAWING S501 FOR STEEL COLUMN SCHEDULE.
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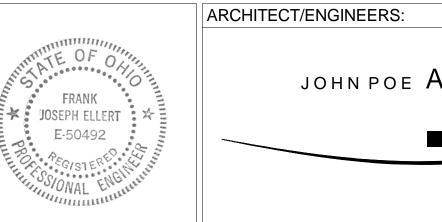
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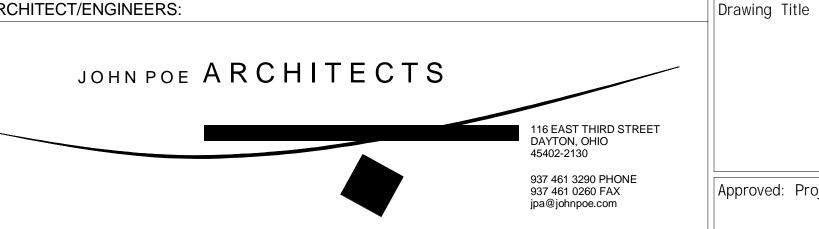




one-eighth inch = one foot

1 Bid Documents







- W12X16

- W12X16

W12X16 (8) V=15K

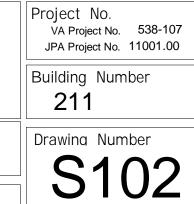
W12X16 (8) V=15K

W14X22 (8) V=15K

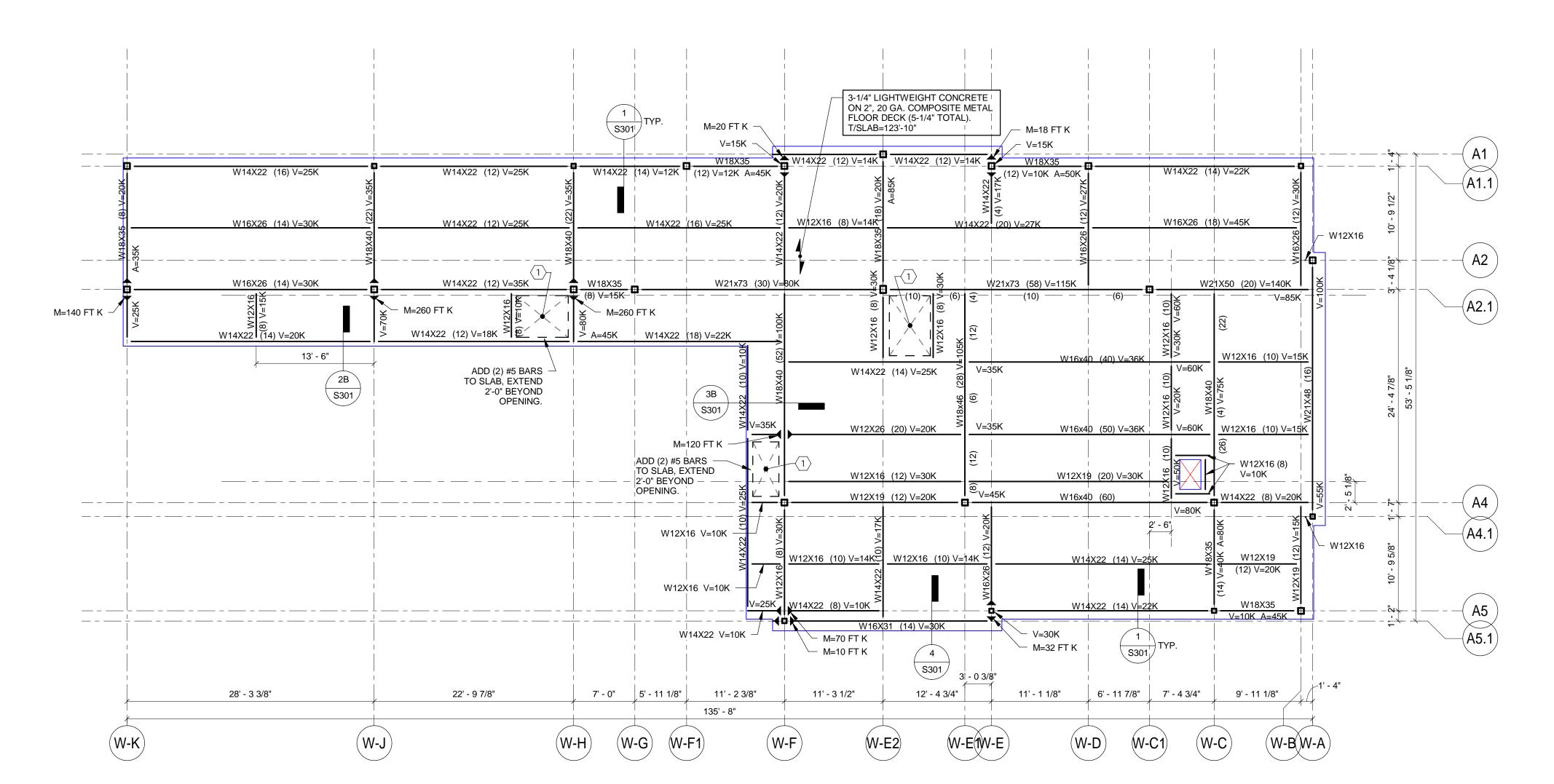
W18X35



Renovate CLC



Office of Construction and Facilities Management



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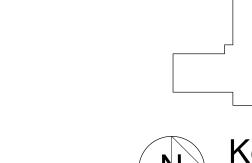
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KeyPlan

Scale: N.T.S.

EXISTING BUILDING

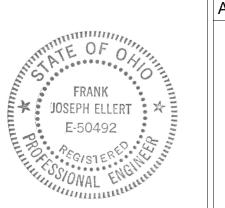
NORTH

Project Title



CONSULTANTS:

9/6/13 Date



1 ATTIC LEVEL FRAMING PLAN

1/8" = 1'-0"



ATTIC LEVEL FRAMING PLAN

Renovate CLC **Building 211 West**

Building Number 211 Chillicothe, Ohio Drawing Number **S103**

Project No.

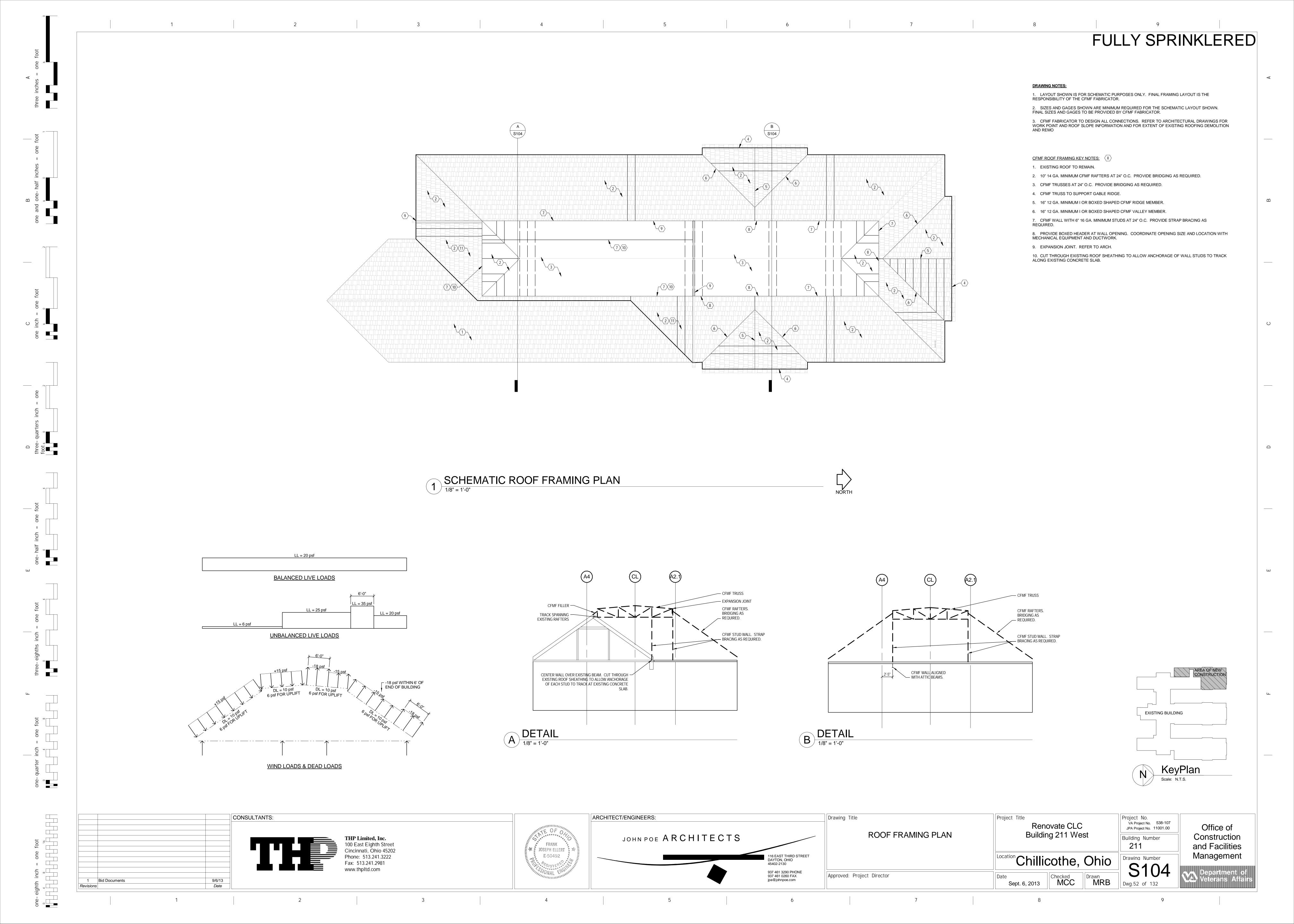
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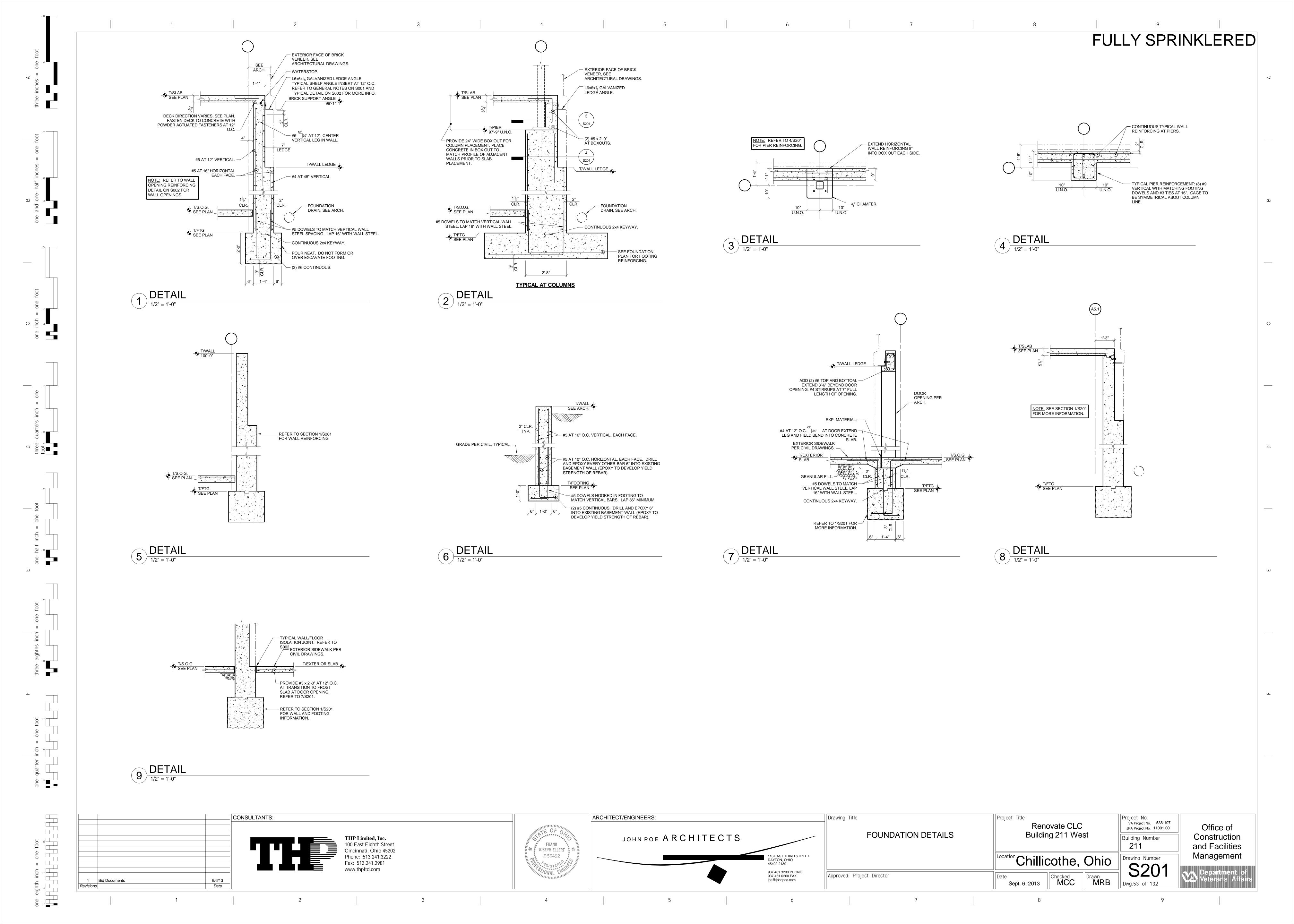
JPA Project No. 11001.00 Office of Construction and Facilities Management

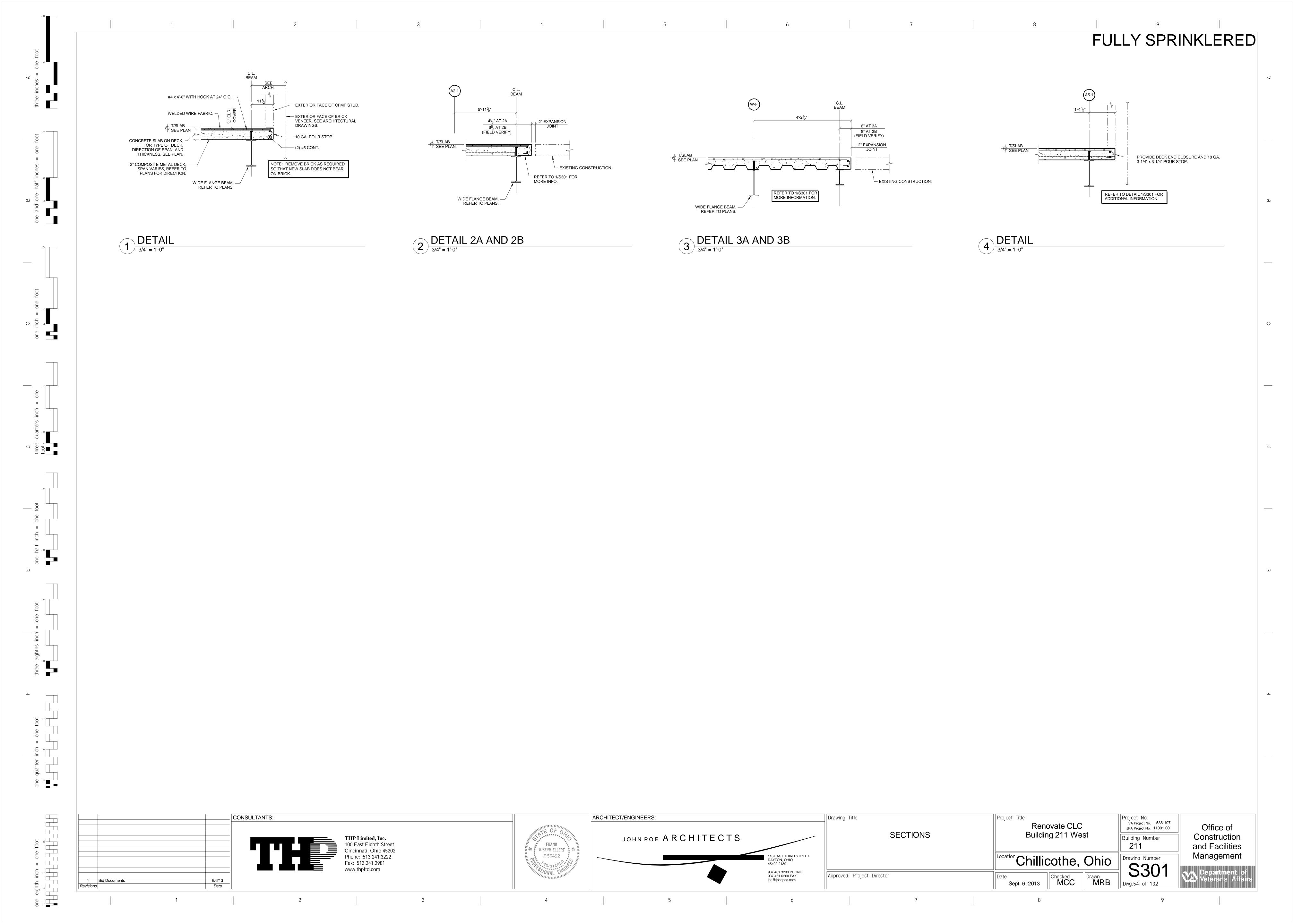
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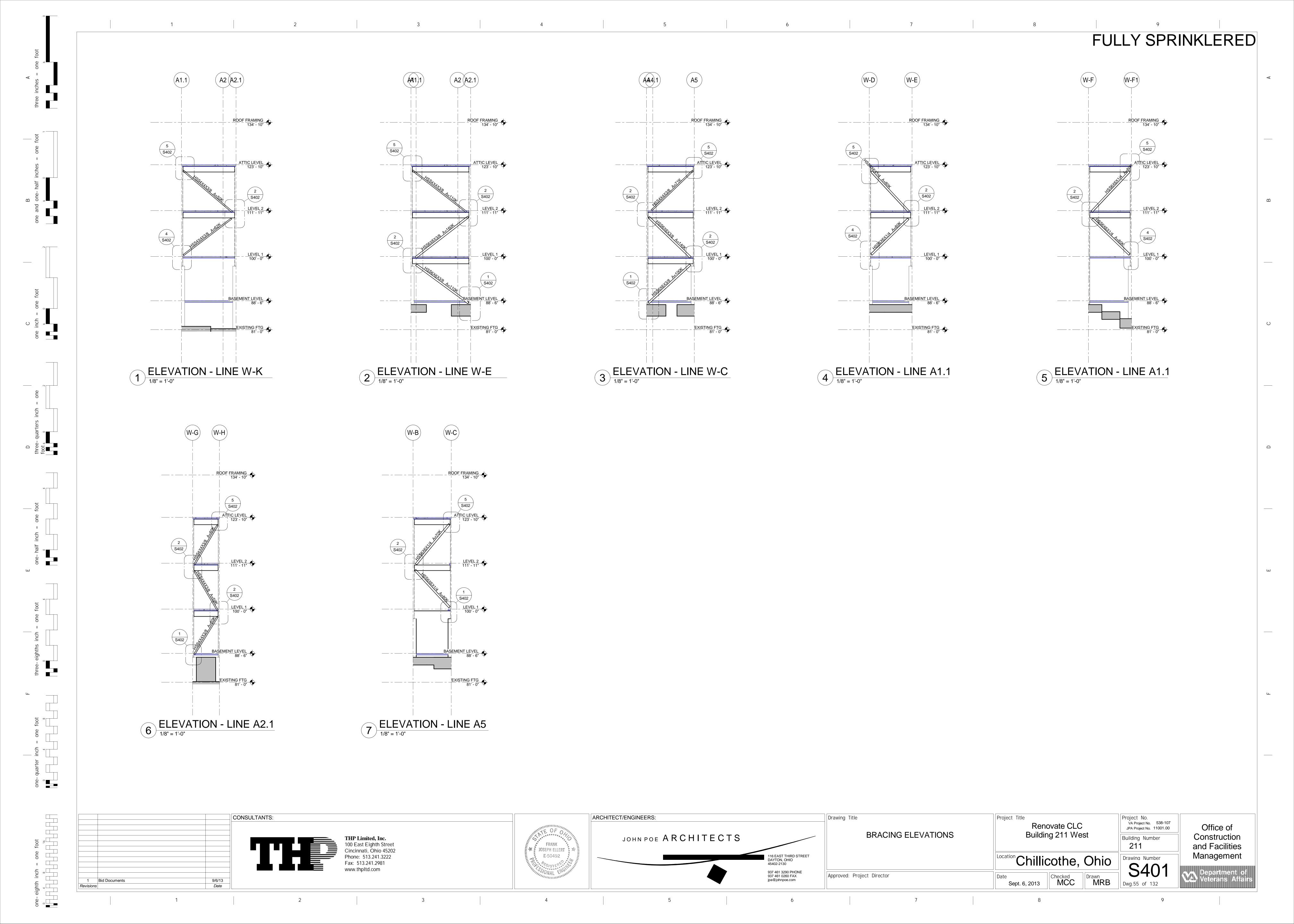
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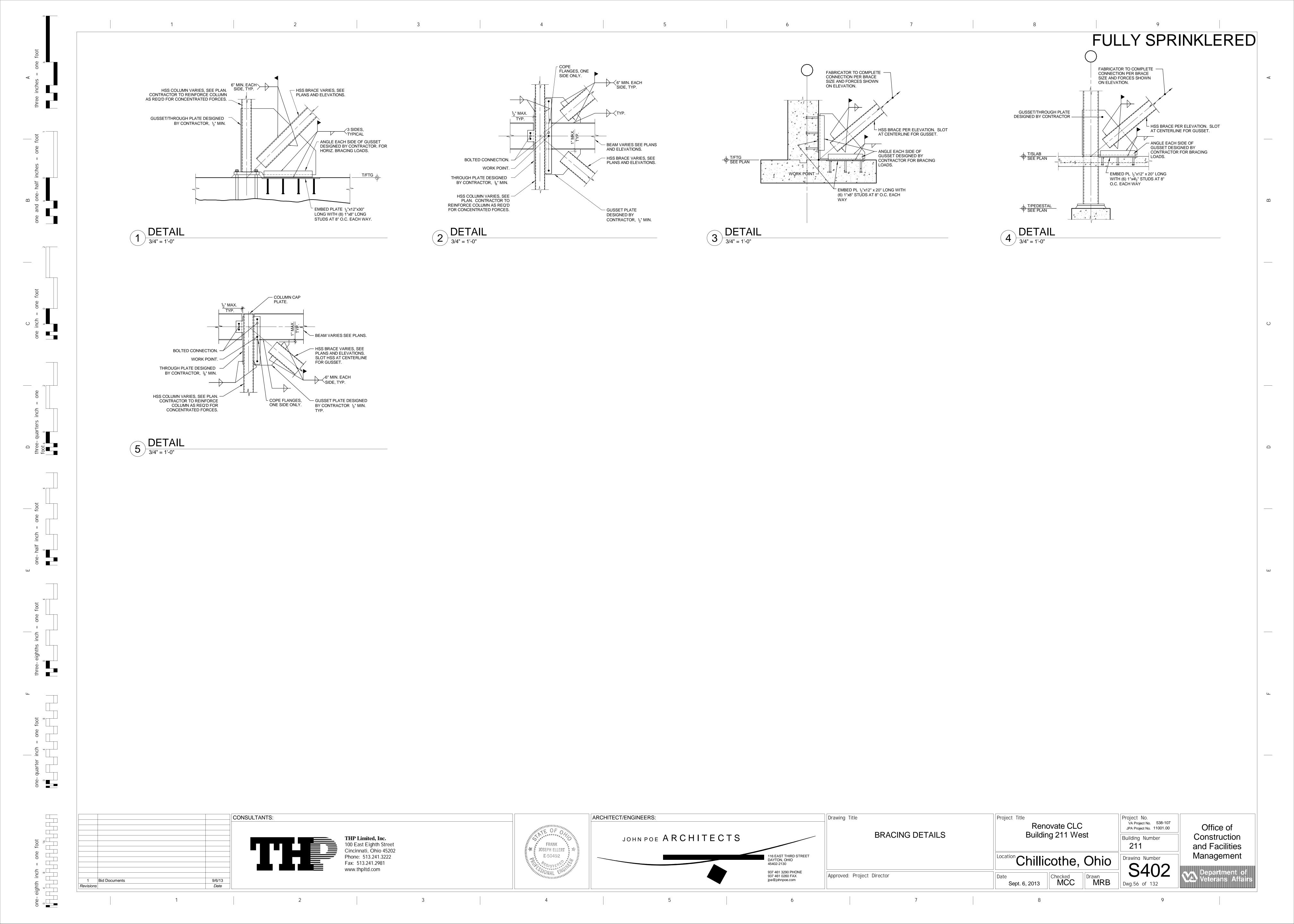
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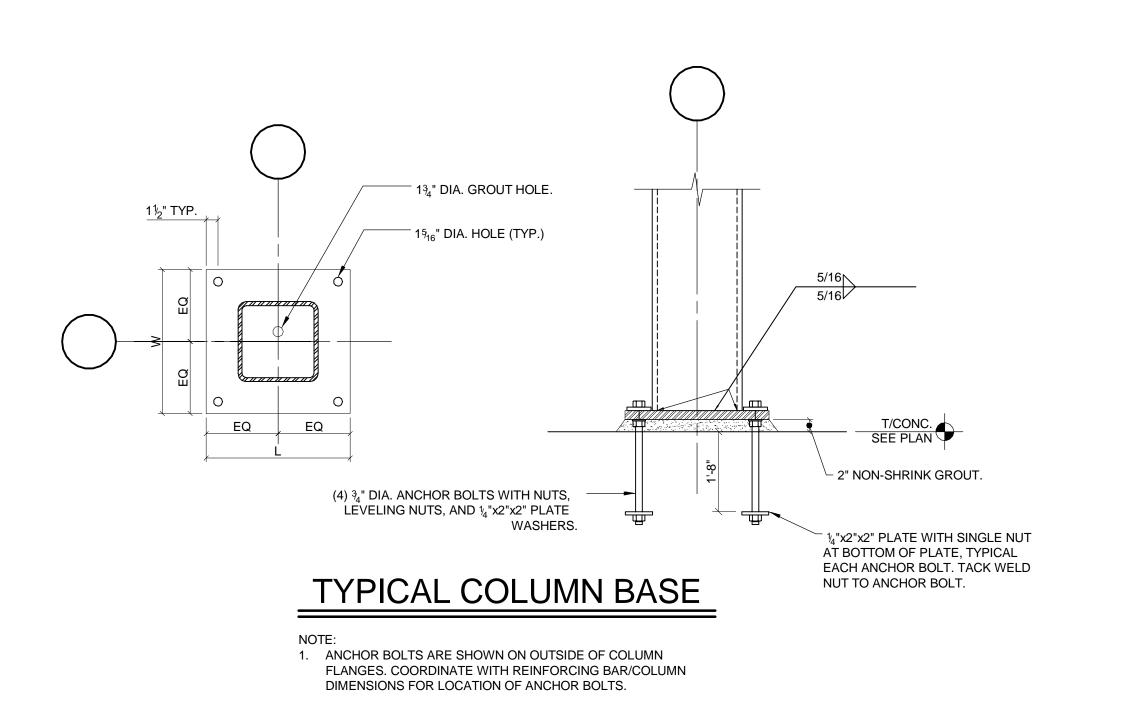


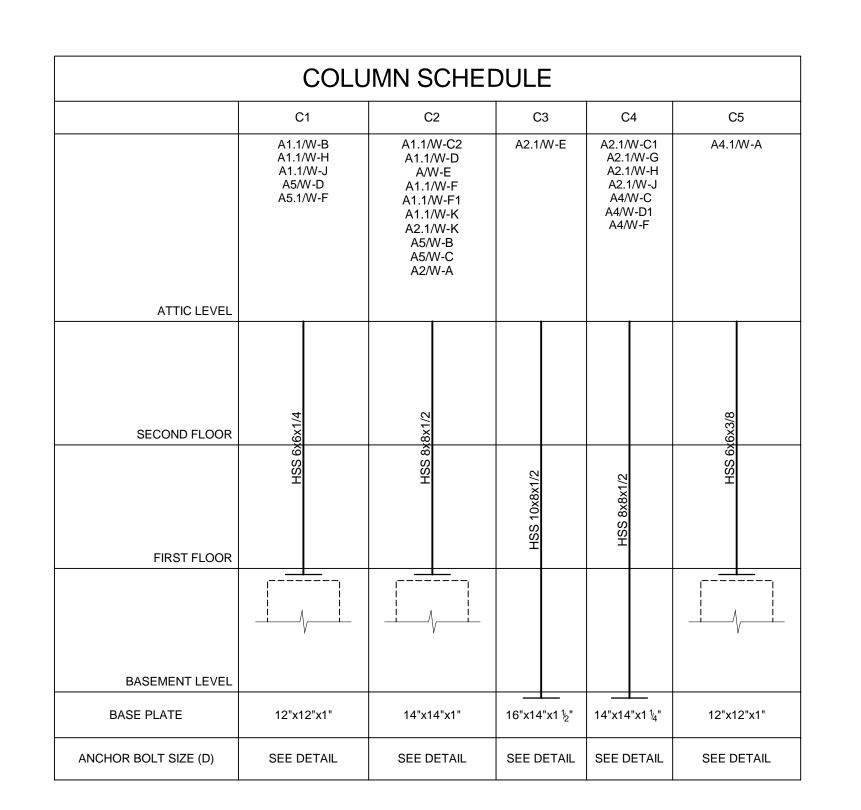


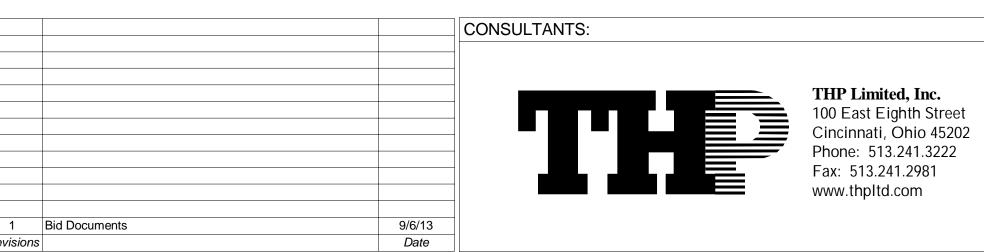






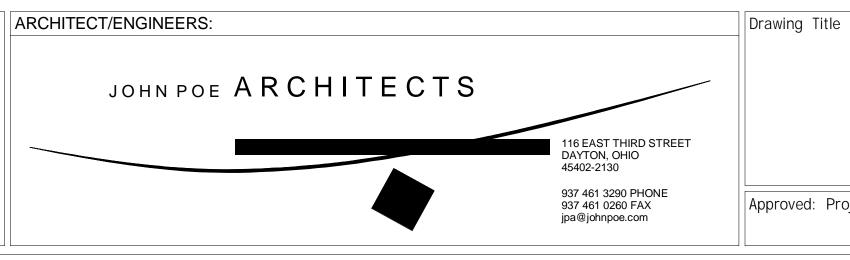


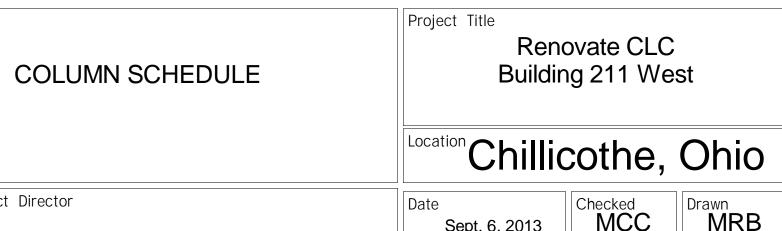


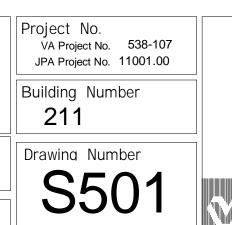


one- eighth inch = one foot









Office of Construction and Facilities Management

Approved: Project Director

Department of Veterans Affairs

